

EFFECTS OF BED-SEDIMENT GRAIN-SIZE DISTRIBUTION ON WAVE HEIGHT PREDICTIONS IN THE ENGLISH CHANNEL

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Abstract

The present study examines the energy dissipation of wind-generated surface-gravity waves by bottom friction in relation to the seabed roughness magnitude. The investigation is based on the phase-averaged numerical wave model SWAN (Simulating WAVes Nearshore) implemented in the English Channel. Two formulations of bottom drag coefficients are considered: the JONSWAP expression and the eddy-viscosity model of Madsen. Numerical predictions of significant wave height are compared with available field data collected at (1) three offshore lightships and (2) two nearshore wave buoys off Cherbourg and Le Havre harbours. Best estimates are found with the heterogeneous bottom roughness associated with the grain size of seabed sediments. Mappings of coastal areas influenced by bottom friction are produced exhibiting significant energy dissipation in the Normano-Breton Gulf and the surroundings of the Isle of Wight.

Key words: modelling, SWAN, bottom friction, English Channel, Le Havre, Cherbourg.

1. Introduction

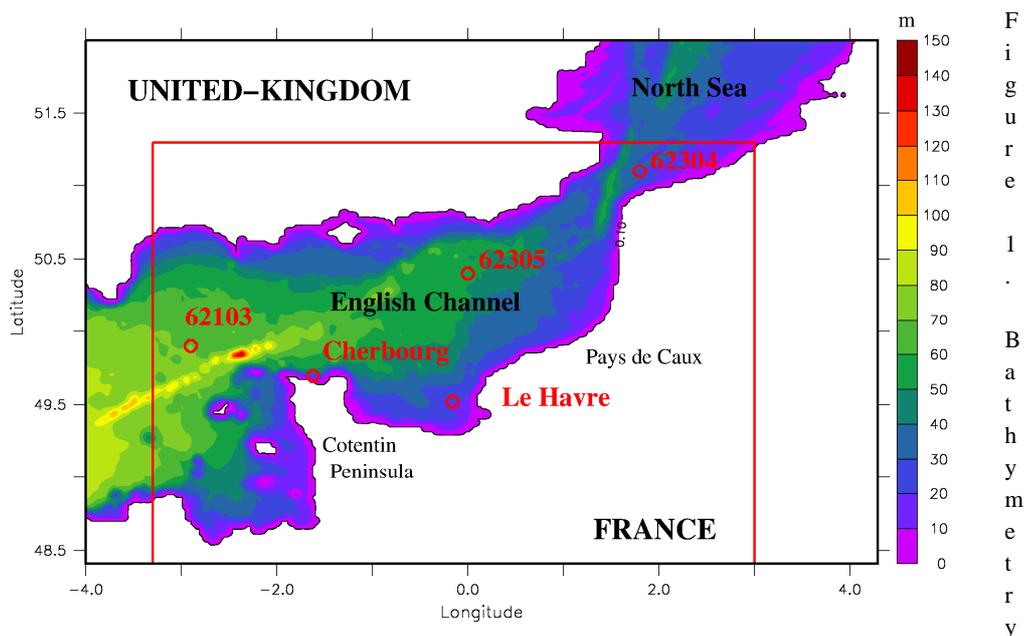
Wind-generated surface-gravity waves propagating in shallow water may experience significant energy dissipation due to seabed interactions when the orbital motions of the water particles extend down to the seabed (*e.g.*, Johnson and Kofoed-Hansen, 2000). Neglecting this energy loss is liable to result in substantial overestimation of nearshore wave height with consequences in the quantification of sediment transport rates or the design of coastal structures (Work *et al.*, 2004). During extreme storm events such as hurricanes, this dissipation of wave energy is likely to decrease by 40-50 % the significant wave height along the coastline (*e.g.*, Riedel *et al.*, 2005; Siadatmousavi *et al.*, 2011).

Bottom friction is considered to be the dominant mechanism of wave-bottom interaction in many continental shelves with fine sandy bottom of median diameter d_{50} in the range of 0.1 – 0.4 mm (*e.g.*, Shemdin *et al.*, 1978; Bertotti and Cavaleri, 1994). In the action or energy balance equation of third-generation wave models, it is generally expressed with the formulation proposed by Weber (1991a, b). The key point of this mathematical expression is the parameterisation of a dissipation coefficient C_b on the basis of hydrodynamics and bed sediment properties. Numerous formulations have been proposed to approach this coefficient (see Luo and Monbaliu, 1994 or the WISE group, 2007 for a detailed review). The simplest method commonly used in operational prediction models consists in prescribing constant empirical values on the basis of the JONSWAP parameterisation adopted by Hasselmann *et al.* (1973) and reviewed later by Bouws and Komen (1983). Although this approach performs well in a number of idealised test cases with wind-wave conditions (Tolman, 1994), more complicated models have been developed to integrate the effects of hydrodynamics and sediment properties (*e.g.*, Hasselmann and Collins, 1968; Collins, 1972; Madsen *et al.*, 1988; Weber, 1991a,b). Given the lack of information on bottom materials, numerous investigations were dedicated to comparisons of dissipation coefficients formulations in relation with sea roughness and wind-drag coefficient (*e.g.*, Luo and Monbaliu, 1994; Johnson and Kofoed-Hansen, 2000; Padilla-Hernandez and Monbaliu, 2001; Zijlema *et al.*, 2012). The nature and properties of bottom sediments were considered in few applications (*e.g.*, Arduin *et al.*, 2003a, b; Siadatmousavi *et al.*, 2011). The purpose of the present study is to evaluate the effects of bed-sediment grain-size distributions on

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predicted significant wave height in the English Channel (Fig. 1). This shelf environment is characterised by a highly heterogeneous spatial distribution of bottom sediments with (1) very fine sands, silts and muds in bays and estuaries and (2) pebbles and gravels in the Dover Strait, off the “Pays de Caux” and over an extensive zone in the Central Channel between the Isle of Wight and the Cotentin Peninsula (*e.g.*, Vaslet *et al.*, 1979; Larssonneur *et al.*, 1982). Furthermore, it is occasionally subjected to storm events.

The approach retained here relies on a comparison between numerical predictions and observations. Field measurements are realised at three offshore lightships and two nearshore wave buoys off Cherbourg and Le Havre harbours (Fig. 1, Section 2.1). Modelling is based on the phase-averaged wave model SWAN (Simulating WAVes Nearshore) (Booij *et al.*, 1999) modified to account for a new parameterisation of wind drag coefficient proposed by Zijlema *et al.* (2012) at high wind speeds (Sections 2.2 and 2.3). A pre-processing module computes the required surficial sediment granulometric distribution from a series of bottom samples and the associated heterogeneous frictional parameters. Numerical predictions are compared with observations over the period from December 2007 to March 2008 characterised by an extreme storm event on 10 March 2008 with significant wave height over 10 m at the western entrance of the English Channel (Section 3.1). The role of bottom friction on wind waves is evaluated comparing numerical predictions obtained with the formulations of wind-drag coefficients proposed by Wu (1982) and Zijlema *et al.* (2012) (Section 3.2). The effects of bottom friction on the dissipation of wave energy are investigated considering the JONSWAP expressions proposed by Hasselmann *et al.* (1973) and Bouws and Komen (1983) and the eddy-viscosity model of Madsen *et al.* (1988) to integrate the hydrodynamics and heterogeneous distribution of bottom-sediment grain size (Section 3.3). Finally, mappings of numerical predictions are produced over the entire computational domain to encompass the spatial and temporal changes of (1) the significant wave height and (2) bottom dissipation coefficient C_b with respect to constant default values (Section 3.4).



of the computational domain with the locations of the measurements points. The red rectangle shows the area where the spatial heterogeneous bottom roughness is introduced.

2. Materials and Methods

2.1. Experiment description

Wave measurements here used were obtained at the three offshore lightships 62103 ($\lambda=-2.90^\circ\text{W}$, $\varphi=49.90^\circ\text{N}$), 62305 ($\lambda=0^\circ\text{W}$, $\varphi=50.40^\circ\text{N}$) and 62304 ($\lambda=1.80^\circ\text{W}$, $\varphi=51.10^\circ\text{N}$) of the UK Meteorological

Office and the two nearshore wave buoys off Le Havre ($\lambda=-0.16^\circ\text{W}$, $\varphi=49.52^\circ\text{N}$) and Cherbourg ($\lambda=-1.62^\circ\text{W}$, $\varphi=49.70^\circ\text{N}$) harbours of the French CANDHIS database (Centre d'Archivage National de Données de Houle In Situ) (Fig. 1). The instrumentation network is deployed in water depths ranging from 70 m at lightship 62103 to 20 m at the wave buoy of Le Havre.

2.2. Model description

SWAN solves the time-dependent spectral action balance equation. It accounts for depth shoaling, refraction, non-linear transfer of energy through wave-wave interactions and wave-energy dissipation due to whitecapping, bottom friction and depth-induced breaking. The modelling procedure ignored the modifications of waves components by the current and the free-surface elevation. The parameterisation adopted for sources and sinks are briefly detailed hereafter. Energy dissipation in random waves due to depth-induced breaking is quantified according to Battjes and Janssen (1978).

The sink term of bottom friction is computed according to the following formulation:

$$S_{ds,b} = -C_b \frac{\sigma^2}{g^2 \sinh^2 kd} E(\sigma, \theta) \quad (1)$$

where k is the wave number of the spectral component, σ is the relative radian frequency equals to the absolute radian frequency in the absence of ambient current, θ is the direction of the wave component, d is the water depth, g is the acceleration of gravity taken equal to $g=9.81 \text{ m s}^{-2}$ and $E(\sigma, \theta)$ is the two-dimensional frequency spectrum. Two formulations are considered for the computation of the bottom friction coefficient C_b . The first parameterisation here retained consists in prescribing constant empirical values. In the JONSWAP experiment, Hasselman *et al.* (1973) suggested to use a value of $C_b=0.038 \text{ m}^2\text{s}^{-3}$ for swell dissipation over sandy bottoms. This value was re-examined by Bouws and Komen (1983) for fully developed wind-seas as $C_b=0.067 \text{ m}^2\text{s}^{-3}$. The second parameterisation follows Madsen *et al.* (1988) who derived a formulation which integrates the effects of wave conditions and bed roughness. The bottom friction coefficient is given by:

$$C_b = f_\omega \frac{g}{\sqrt{2}} U_{rms} \quad (2)$$

where U_{rms} is the root mean square bottom-orbital velocity. f_ω is the non-dimensional friction factor estimated with the formulation proposed by Jonsson (1966):

$$\frac{1}{4\sqrt{f_\omega}} + \log_{10} \left(\frac{1}{4\sqrt{f_\omega}} \right) = m_f + \log_{10} \left(\frac{a_b}{k_n} \right) \quad (3)$$

where m_f is taken equal to $m_f=-0.08$ according to Jonsson and Carlsen (1976), a_b is the representative near-bottom excursion amplitude and k_n is the bottom roughness length scale. This formulation is retained when the condition $a_b/k_n > 1.57$ is satisfied. For values $a_b/k_n < 1.57$, f_ω is taken equal to $f_\omega=0.3$. Whereas numerous algorithms have been developed to integrate the effects of wave ripples generation in numerical wave models (*e.g.*, Graber and Madsen, 1988; Tolman, 1994; Arduin *et al.*, 2003a; Smith *et al.*, 2011), the present investigation is restricted to the consideration of current ripples mainly because of the strong influence of tidal currents on sediment transport in the English Channel (*e.g.*, Guillou *et al.*, 2009; Guillou and Chapalain, 2010). As this study constitutes a first estimate of the effects of the heterogeneous spatial distribution of bottom-sediment grain-size on wave conditions, more complex predictors of sand ripple geometry under the combined influence of tidal current and waves (*e.g.*, Li and Amos, 1998, 2001) are ignored. Bed features are considered over sandy bottom with median diameter $d_{50} < 800 \mu\text{m}$ (Soulsby, 1997). The associated roughness length scale is parametrised according to Yalin (1985) and Wooding *et al.* (1973) resulting in value of k_n around $k_n \sim 240d_{50}$. This relationship is consistent with the linear formulation proposed by Siadatmousavi *et al.* (2011): $k_n=200d_{50}$. Seabeds made of coarser sediments (*i.e.*, $d_{50} > 800 \mu\text{m}$) remain featureless with a roughness length scale equals to $k_n=3d_{90}$ where d_{90} is the grain diameter for which 90 % of the grains by mass is finer.

Wave growth by wind is computed with the exponential term of Komen *et al.* (1984). Two formulations are considered for the wind drag coefficient: (1) the default value proposed by Wu (1982) and (2) the

relationship derived by Zijlema *et al.* (2012) to fit a compilation of observations at high wind speeds. Finally, processes of whitecapping are described with the pulse-model equation of Hasselmann (1974). The wave action balance equation is expressed in a spherical coordinate system. Further details about the mathematical expressions of sources and sinks are available in SWAN technical documentation (SWAN team, 2009).

2.3. Model setup

SWAN is set up on a computational domain covering the English Channel between longitudes -4.000°W and 4.291°W and latitudes 48.410°N and 51.992°N (Fig. 1). It is discretised on a 280×200 horizontal grid with a spacing of 2 km. The model runs with 30 exponentially spaced frequencies ranging from 0.05 to 1 Hz, 60 evenly spaced directions (6° resolution) and a time step of 10 min. Wind velocity components at 10 m above the free surface are provided by the meteorological model ALADIN (“Aire Limitée, Adaptation dynamique, Développement InterNational”, Bénard, 2004, Météo-France). SWAN is driven by the wave components predicted along each open boundary from a regional run of WaveWatch III on the North-Western European continental shelf (F. Ardhuin, Institut Français de Recherche pour l'Exploitation de la MER, France, personal communication issued from the NORGAS configuration, 2010). The heterogeneous spatial distribution of the median diameter d_{50} is computed in an inner domain extended in longitude from 3.300°W to 3.000°E and in latitude from 48.410°N to 51.300°N (Fig. 1). The statistical mixed SFA-kriging (Spherical Factor Analysis) interpolation method proposed by Leprêtre *et al.* (2006) is applied to a series of 2318 bottom sediment samples collected in the framework of the “RCP 378 Benthos de la Manche” program (Cabioch *et al.*, 1977). Further details about the application of this method in the English Channel are available in Guillou (2007) and Guillou and Chapalain (2010). The resulting spatial heterogeneous bottom roughness length scale is displayed in Figure 2. When the formulation of Madsen *et al.* (1988) is retained, k_n is set to its default value $k_n=0.05$ m (SWAN team, 2009) outside of this inner domain.

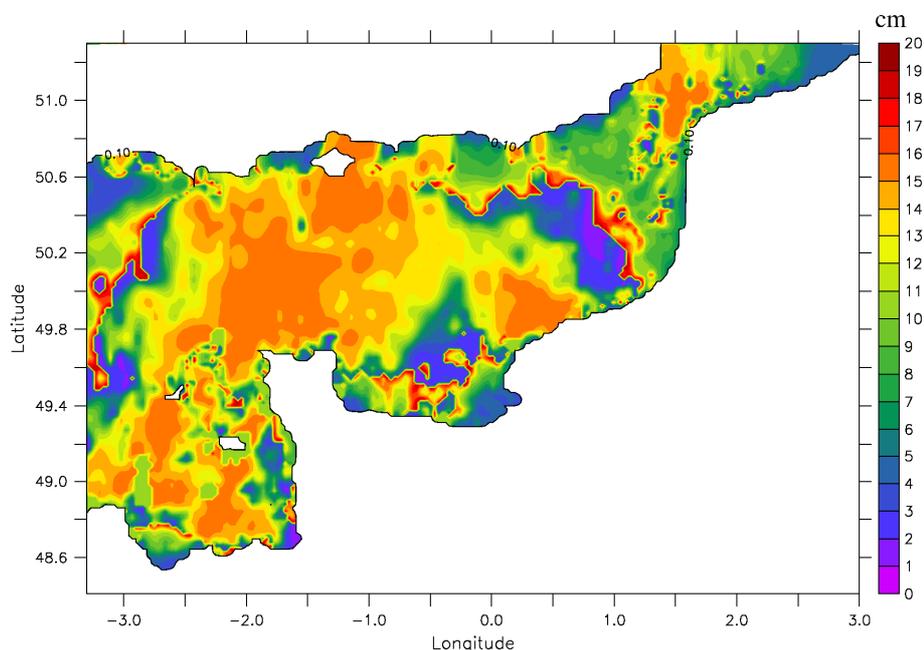


Figure 2. Spatial heterogeneous bottom roughness length-scale k_n in the inner computational domain.

SWAN is run over the period from December 2007 to March 2008 when continuous observations of wave conditions in winter were available at the five measurements sites (Fig. 3). This period is characterised by the succession of six major storm events of averaged significant wave height of 3.5 m in the central English Channel. It presents also an extreme storm event on 10 March 2008 with wave height exceeding 10 m at the western entrance of the English Channel. In order to investigate the effect of the sensitivity of

model predictions to the formulations of wind-drag coefficient, four numerical experiments titled A1 to A4 are conducted (Table 1). The influences of bottom-friction formulations is evaluated on the basis of five experiments titled B1 to B3b (Table 2). These numerical experiments are compared with the standard statistical parameters of the averaged and maximum absolute differences and the relative error or index of agreement introduced by Willmott (1981) as

$$re = 1 - \frac{\sum_i |y_i - x_i|^2}{\sum_i (|y_i - \bar{X}| + |x_i - \bar{X}|)^2} \quad (4)$$

where (x_i) and (y_i) represent the two sets of measured and simulated wave heights and \bar{X} is the averaged of the observations (x_i) over the time period considered. The relative error varies between 0 and 1. It equals to unity for perfect agreement.

Table 1. List of numerical experiments for the estimation of wind-drag formulations.

Numerical experiments	Bottom drag formulations		Wind-drag formulations	
	JONSWAP $C_b=0.067 \text{ m}^2.\text{s}^{-3}$	JONSWAP $C_b=0.038 \text{ m}^2.\text{s}^{-3}$	Wu (1982)	Zijlema <i>et al.</i> (2012)
A1	X		X	
A2		X	X	
A3	X			X
A4		X		X

Table 2. List of numerical experiments for the estimation of bottom-drag formulations with the wind-drag formulation proposed by Zijlema *et al.* (2012).

Numerical experiments	B1	B2a	B2b	B3a	B3b
Formulation of the bottom dissipation coefficient	No friction	JONSWAP $C_b=0.038 \text{ m}^2.\text{s}^{-3}$	JONSWAP $C_b=0.067 \text{ m}^2.\text{s}^{-3}$	Madsen <i>et al.</i> (1988) $k_N=0.05 \text{ m}$	Madsen <i>et al.</i> (1988) $k_N=f(d_{50})$

3. Results and Discussion

3.1. Comparisons with point measurements

Figure 3 displays the observed and predicted from configuration A1 time series of the significant wave height at offshore and nearshore measurement points in December 2007 – March 2008. The model reproduces fairly well the magnitude of the storms heights of early December, January and March 2008 at the five sites of observation. Best estimates are obtained at the wave buoy of Le Havre with a relative error of 0.96. Predictions tend however to slightly overestimate measurements by 5-10 % at lightships 62103 and 62305 and the wave buoy of Cherbourg. This tendency appears in particular during the storms of January 2008. Numerical results at lightship 62304 tend however to underestimate observations. Whereas these predictions present the most important differences with measurements, the relative error stands over 0.88.

3.2. Sensitivity to wind-drag formulation

The relative errors for the numerical experiments A1 to A4 are displayed in Figure 4. Best estimates of the significant wave height are globally obtained with the formulation of Zijlema *et al.* (2012) which presents

better agreement with observations of wind-drag coefficient than the expression of Wu (1982).

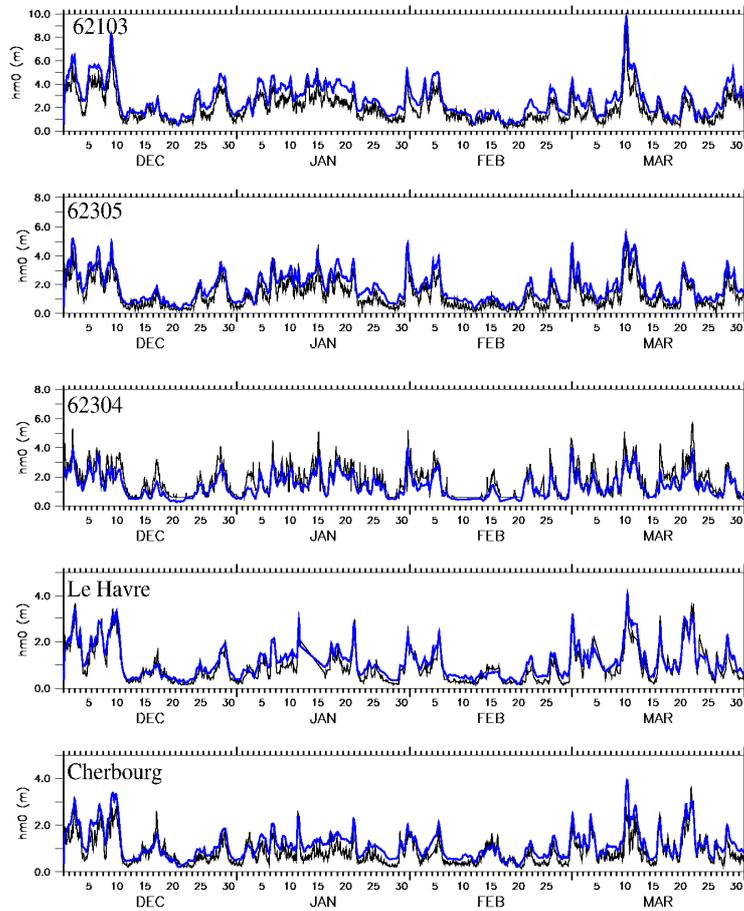
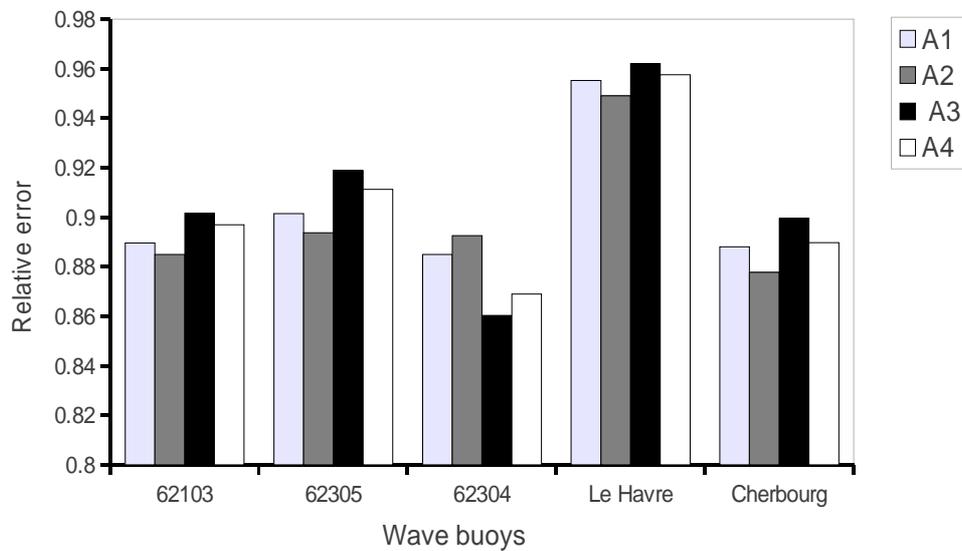


Figure 3. Time series of the (black line) observed and (blue line) predicted (from configurations A1) significant wave height at offshore and nearshore wave buoys in December 2007 –March 2008.



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errors in predictions of the significant wave height at measurement points
for the four numerical experiments A1 to A4.

Nevertheless, the new formulation tends to increase the differences at the lightship 62304. Indeed, the wind drag formulation of Zijlema *et al.* (2012) results in lower values than with the original formulation proposed by Wu (1982). During the period of simulation, these differences are exhibited at high wind speeds over 14 m s^{-1} with wind-drag coefficients by 10-30 % higher with the expression of Wu (1982) than with the formulation of Zijlema *et al.* (1982) (Fig. 5). For the same dissipation coefficient by bottom friction, lower significant wave heights are thus obtained with the new formulation. As initial predictions from configuration A1 (Fig. 3) present a global tendency to underestimate measurements at lightship 62304, the expression of Zijlema *et al.* (2012) increases these differences. Nevertheless, the energy balance of the waves at this measurement point appears consistent with conclusions of Zijlema *et al.* (2012) during the Texel storm in the southern North Sea. Best results are obtained with (1) the default wind drag parameterisation of Wu (1982) combined with high bottom friction ($C_b=0.067 \text{ m}^2\text{s}^{-3}$) and (2) the new wind drag of Zijlema *et al.* (2012) with low friction ($C_b=0.038 \text{ m}^2\text{s}^{-3}$). This may be related to stronger wind seas in this area than in the central or western English Channel. For the four remaining measurements sites, best estimates are reached with the new wind-drag formulation and the highest bottom friction proposed by Bouws and Komen (1983).

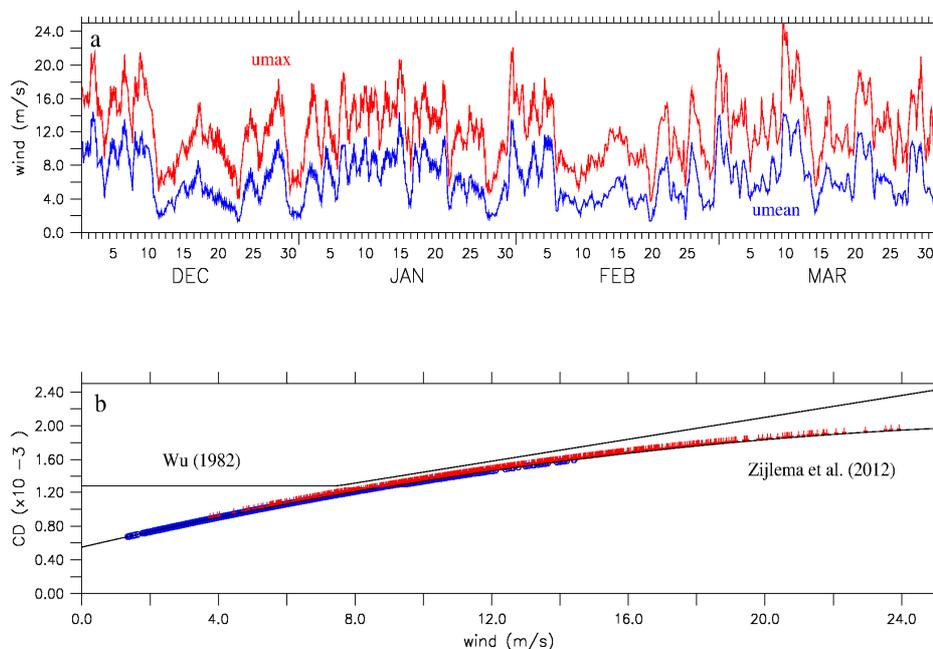


Figure 5. Time series of (a) the averaged and maximum wind speed predicted by ALADIN meteorological model over SWAN computational domain in December 2007-March 2008 and (b) the associated evolution of wind drag coefficients obtained with the formulations of Wu (1982) and Zijlema *et al.* (2012).

3.3. Sensitivity to bottom-friction formulation

The sensitivity of model predictions to bottom-friction formulations is evaluated retaining the expression of Zijlema *et al.* (2012) for the wind-drag coefficient. Figure 6 displays the relative errors for the five numerical experiments titled B1 to B3b at the lightship 62305 and the wave buoys of Le Havre and Cherbourg during the period of interest. Predictions at lightships 62103 and 62304 are not considered here because of the proximity of these measurement points to the boundaries of the inner domain where the heterogeneous roughness length scale is introduced. In spite of a global weak sensitivity of predictions to

bottom friction, comparing predicted time series at the wave buoy of Cherbourg shows that neglecting the energy dissipation by bottom friction results in a local overestimation by 17 % of the significant wave height during the storm of 10 March 2008. Estimates obtained with the constant empirical value of Bouws and Komen (1983) (B2b) are very close to numerical results issued from the formulation of Madsen *et al.* (1988) integrating the default roughness length scale of $k_n=0.05$ m (B3a). This is consistent with the estimation of wave models performance by Tolman (1991) who found satisfactory results for k_n in the range of 2-5 cm. The integration of the grain-size distribution of bottom sediments improves the numerical estimates at the lightship 62305 and the wave buoy of Cherbourg. Nevertheless, predictions at the wave buoy of Le Havre are slightly modified. This may be related to the fact that the surroundings values of k_n displayed in Figure 2 are very close to the uniform value adopted in configuration B3a.

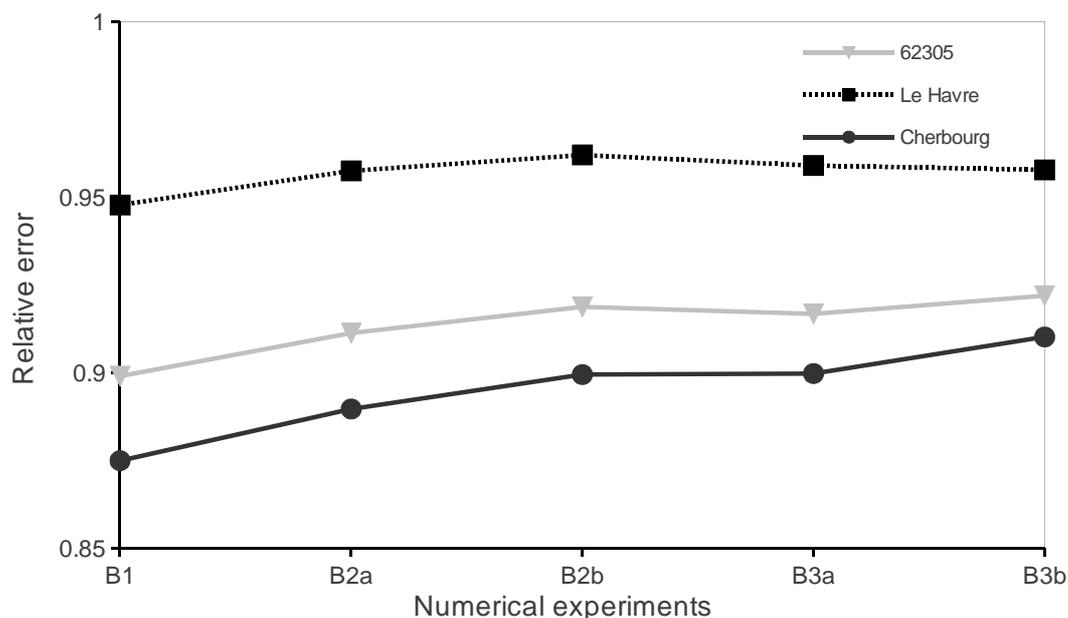


Figure 6. Relative error in predictions of the significant wave height at lightship 62305 and wave buoys of Le Havre and Cherbourg for the four numerical experiments B1 to B3b.

3.4. Mappings of bottom friction effects on wave height

Numerical predictions are exploited further to encompass the spatial and temporal changes of the significant wave height in relation to the integration of the spatial heterogeneous roughness length scale (Fig. 7). Figures 7-b and 7-c confirm the weak sensitivity of measurement points to wave-energy dissipation by bottom friction. The integration of the bed grain-size distribution has an impact in areas of pebbles and gravels exposed to the incoming waves from the North Atlantic Ocean: the Normano-Breton Gulf, the surroundings of the Isle of Wight and the french coastline off the Pays-de-Caux and the southern Dover Strait. Differences are particularly noticeable in the eastern extend of the Isle of Wight reaching the maximum absolute value of 1 m.

Figure 8 displays the spatial distribution of the averaged bottom friction coefficient C_b predicted from configurations B3a and B3b in the inner domain where the grain size of bottom sediments has been introduced. The default formulation of Madsen *et al.* (1988) ($k_n=0.05$ m) results in a bottom friction coefficient between 7 and 20 times greater than the constant empirical value proposed by Bouws and Komen (1983) in the exposed areas of the Normano-Breton Gulf or the surroundings of the Isle of Wight. The integration of the heterogeneous grain size of bottom sediments leads to a global reduction of the dissipation coefficient smoothing the range of predicted values. The averaged bottom friction coefficient is thus varying between 0.04 and 0.09 $m^2 s^{-3}$ in the western English Channel. This weak spatial variability of

predicted values may explain the relative success of constant empirical coefficients of Hasselmann *et al.* (1973) and Bouws and Komen (1983) as exhibited by Tolman (1994).

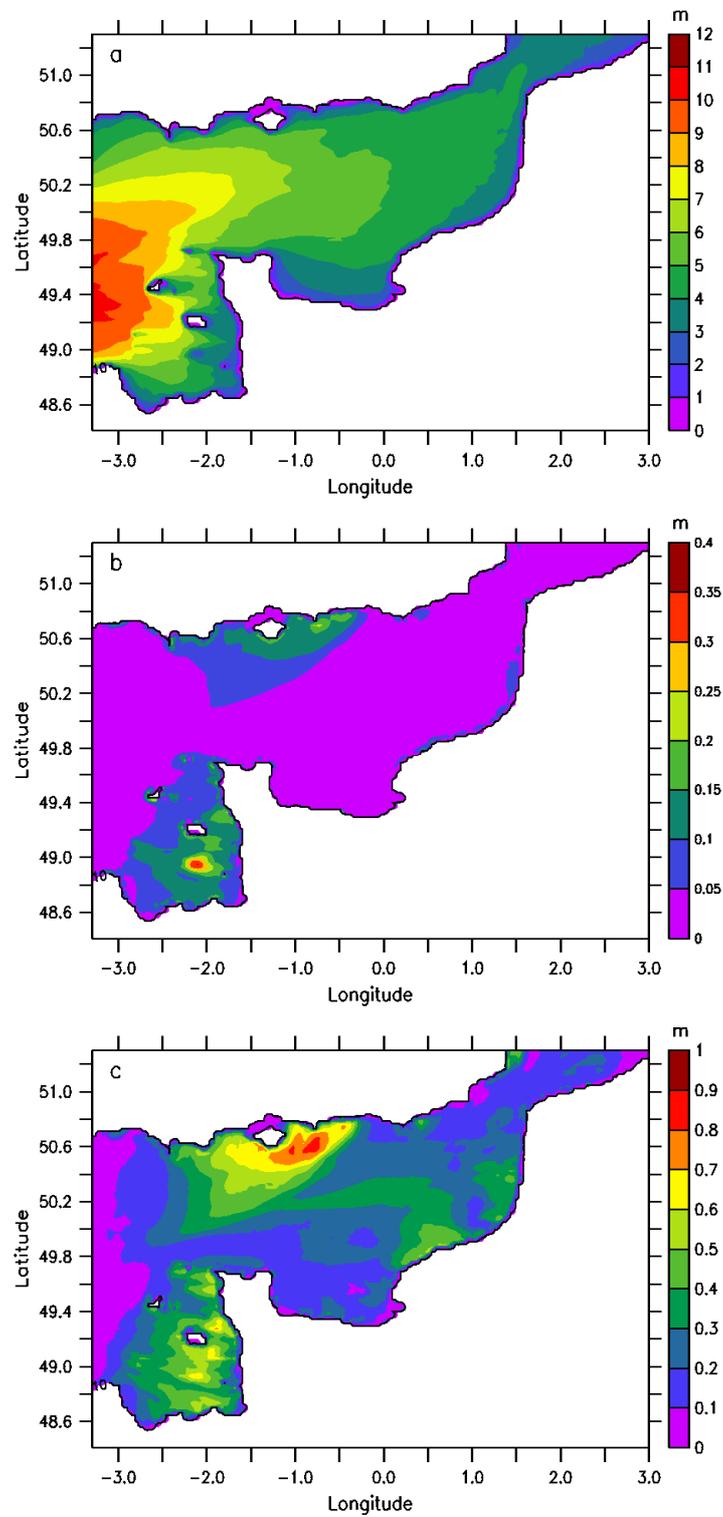


Figure 7. (a) Maximum significant wave height predicted from configurations B3a in December 2007 - March 2008. (b) Averaged and (c) maximum absolute differences

between significant wave height predicted from configurations B3b and B3a.

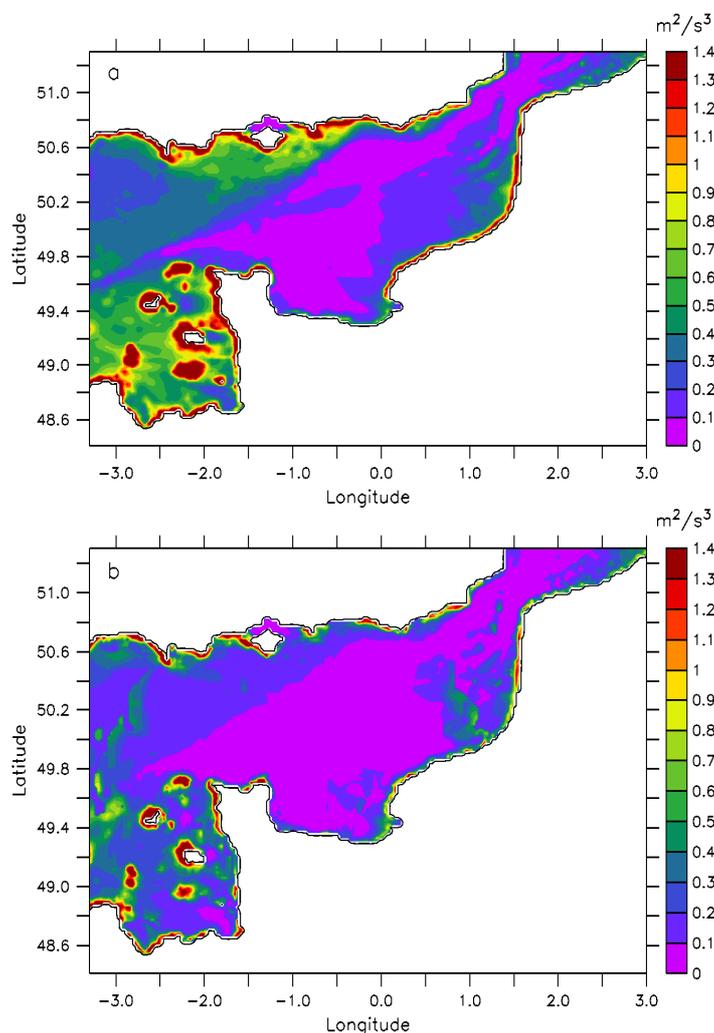


Figure 8. Averaged bottom friction coefficients predicted from configurations (a) B3a and (b) B3b in December 2007 – March 2008.

4. Conclusions

The numerical wave propagation model SWAN has been set up in the English Channel and the southern North Sea to investigate the effects of bed grain-size distributions on predicted significant wave height. Numerical results have been compared with in situ measurements at three offshore lightships and two nearshore wave buoys off Le Havre and Cherbourg harbours. The main outcomes of the present study are the following.

- (1) Numerical predictions appear to be sensitive to the formulation of wind-drag coefficient. Best estimates are globally obtained with the expression recently established by Zijlema *et al.* (2012) to fit a set of observations at high wind speeds.
- (2) Whereas predictions at measurement points show in averaged a slight sensitivity to bottom friction, differences are exhibited during storm events with overestimation of the significant wave height liable to reach 17 % by neglecting the dissipation of wave energy by bottom friction.
- (3) The integration of the grain-size distribution of bottom sediments improves the numerical

estimates at the lightship 62305 and the wave buoy of Cherbourg. Furthermore, the combined effects of hydrodynamics and bed sediment properties in the formulation of Madsen *et al.* (1988) tends to reduce the spatial variability of the averaged dissipation coefficient by bottom friction. This is consistent with the use of constant empirical values as suggested by Hasselmann *et al.* (1973) and Bouws and Komen (1983).

The present study provides thus interesting insights into the effects of bottom friction on wind-generated surface-gravity waves in the English Channel, its dependency with the formulation of wind drag and the parameterisation of bottom roughness length scale. The present study was however restricted to the significant wave height neglecting the effects of the tide. Prospectives of this research will consist in (1) extending the comparison of numerical predictions to the wave period and (2) integrating the tide in waves computation and the generation of waves/current sand ripples. These results will help to improve the computation of nearshore waves condition along the English and French coastlines of the English Channel of utmost interest for numerous coastal engineering applications.

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